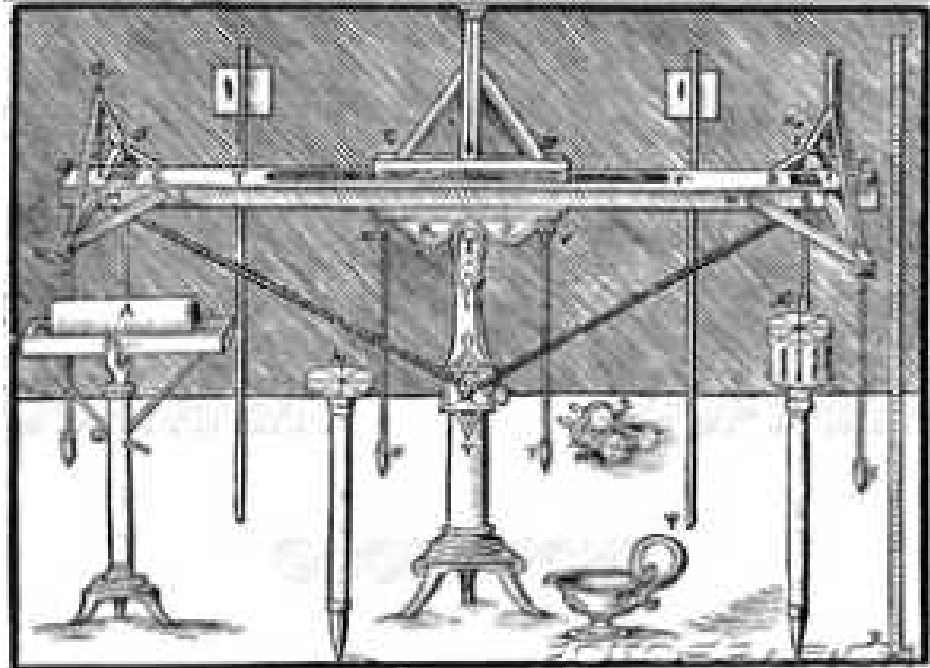


# Surveying & Measurement

Levelling

# Levelling

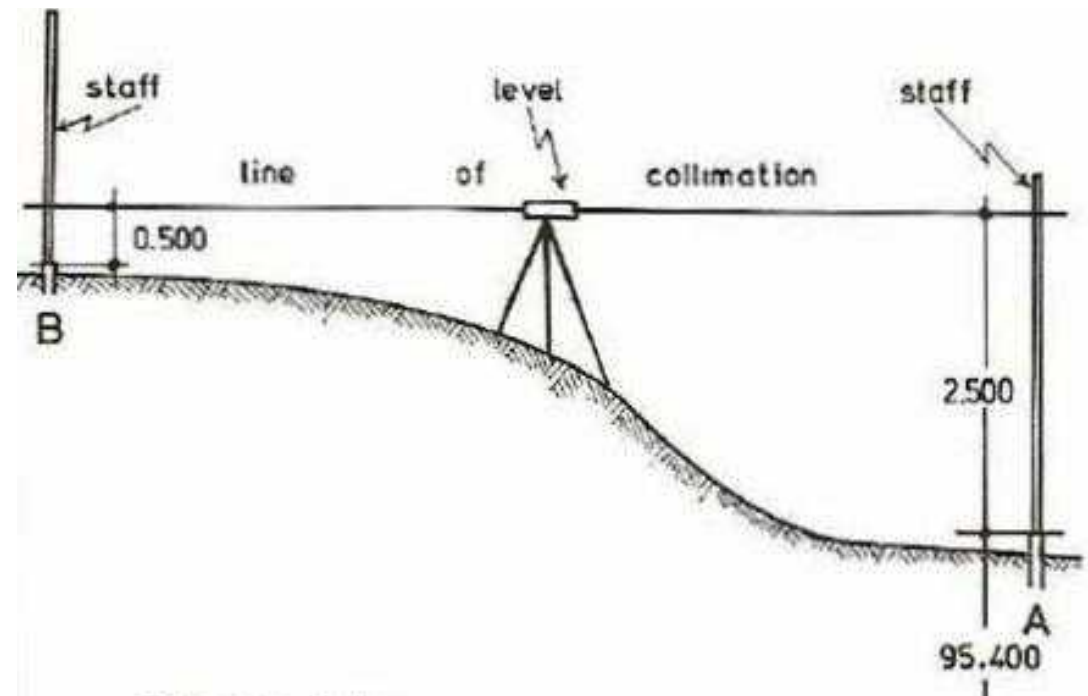
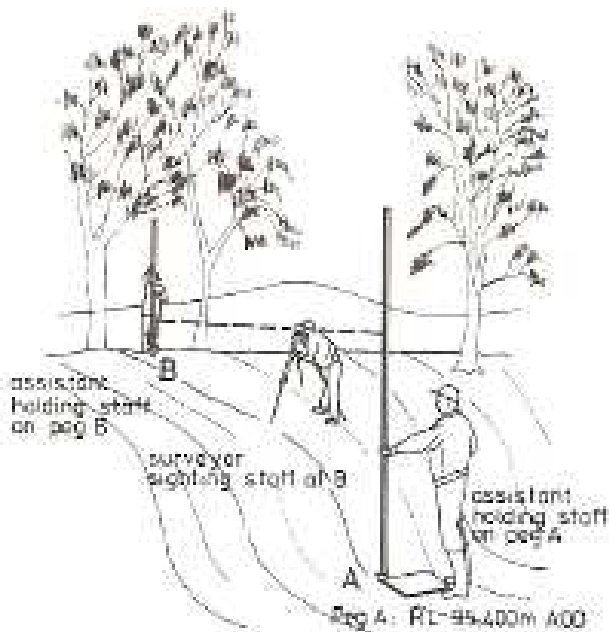
- The general term applied to any of the various processes by which elevations of points or differences in elevation are determined.

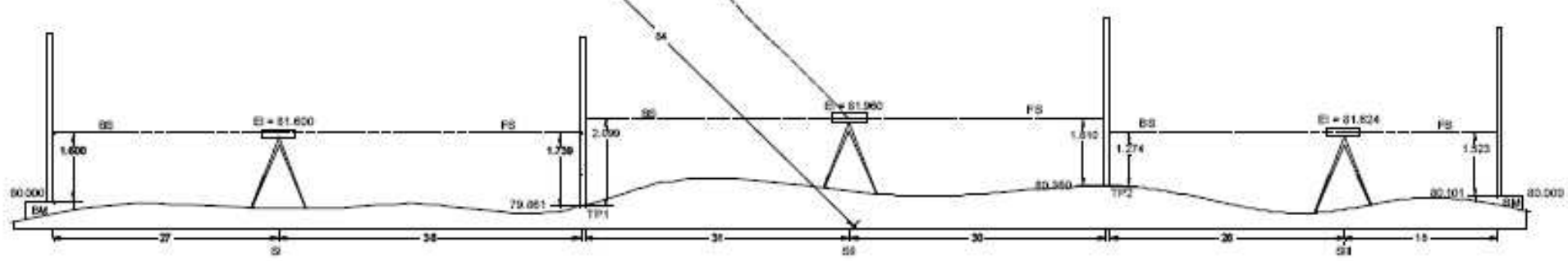
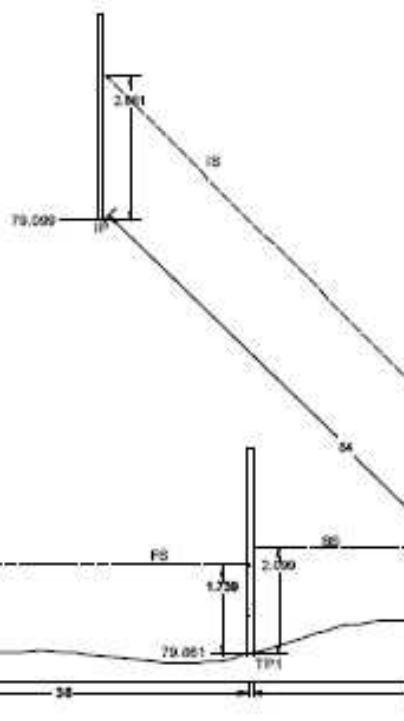
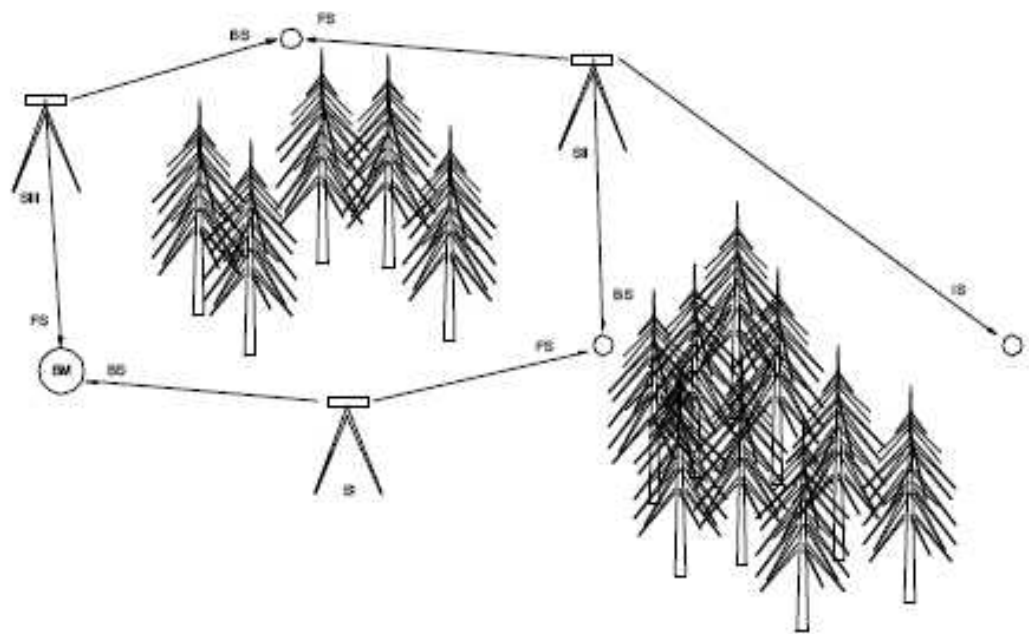


# Levelling

- To measure the difference in height ( $\Delta H$ ) between two points A and B, vertical rods are set up at each of these two points and a level somewhere between them.
- The height difference between A and B is the difference between the rod (staff) readings.

- Once the elevation of a point is determined, that point can be used for determining the elevations of other points.





# Some Definitions <sup>(1)</sup>

- **Levelling** · A surveying operation carried out to determine the elevation of points or to find the difference in elevation of points
- **Spirit Level/Engineer's Level** · A surveying instrument used to carry out levelling
- **Backshot (BS)** · A sighting with a level back to a point of known elevation
- **Foreshot (FS)** · A sighting with a level to determine the elevation of a point
- **Turning Point** · A point at which you have established an elevation with FS and on which you will subsequently take a BS
- **Intermediate Shot** · A foreshot to a point at which you want to know the elevation but which will not be used as a turning point

# Some Definitions (2)

- **Peg Test** · Surveying operation carried out to determine if the levelling bubble and telescope line-of-sight are parallel
- **Elevation of Instrument (EI)** · Elevation of the telescope cross-hairs
- **Balancing shots** · Attempt when doing a levelling survey to keep the lengths of FS and BS at any given instrument setup as close as possible.
- **Closure Error** · Difference in elevation determined from the levelling survey and the known elevation of a benchmark.

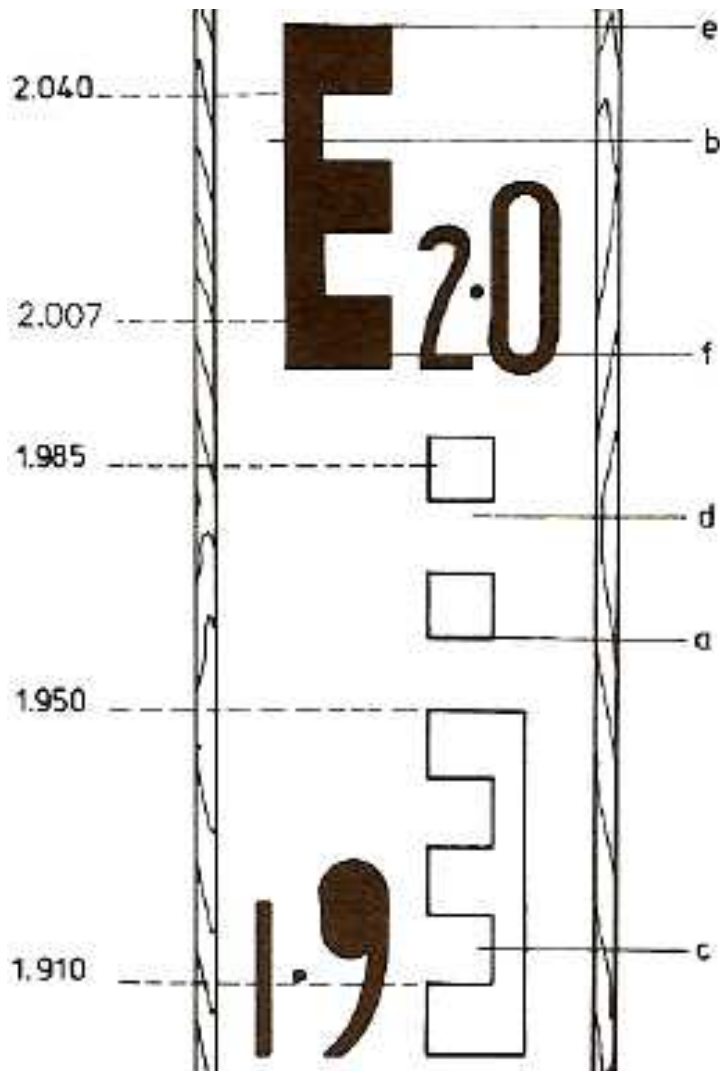
# The Use of Levelling

- To design highways, railroads, canals, sewers, water supply systems etc, having grade lines that best conform the existing topography,
- To lay out construction projects according to planned elevations,
- To calculate volume of earthworks and other materials,
- To investigate drainage characteristics of an area, etc.



# The Equipments :

## Rod (Staff) (1)



### Example

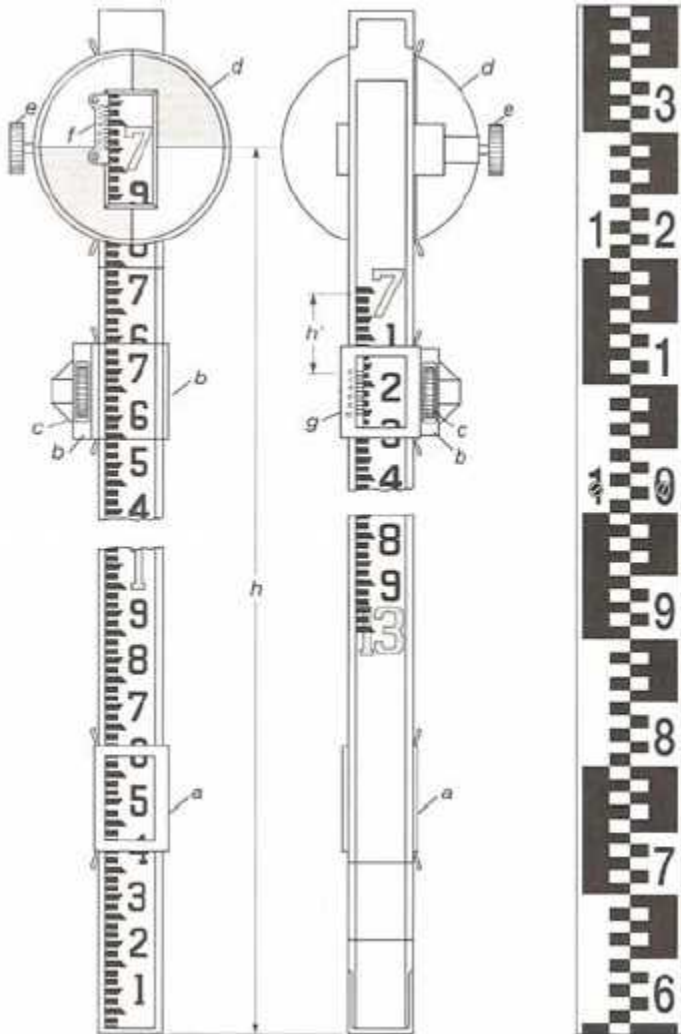
2 Make a list of the staff readings 'a' to 'f' in Fig. 4.14 and reading 'g' in Fig. 4.15.

### Solution

a, 1.960 m	b, 2.033 m	c, 1.915 m
d, 1.978 m	e, 2.050 m	f, 2.002 m
g, 1.156 m		

# The Equipments :

## Rod (Staff) <sup>(2)</sup>

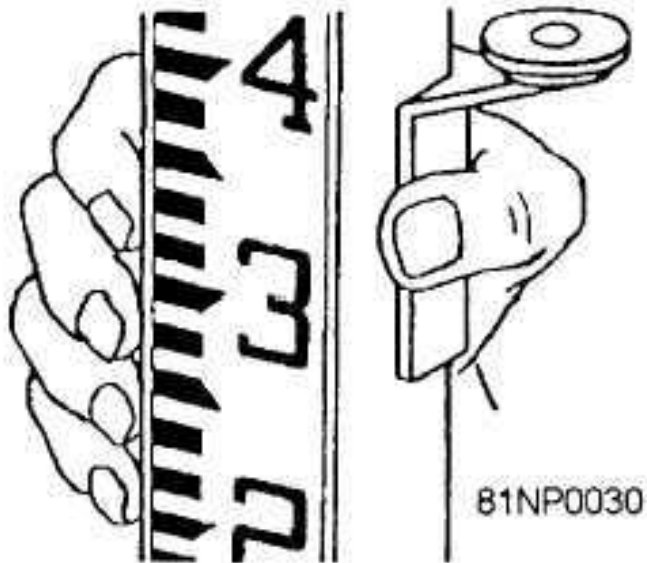


Philadelphia Rod (front)

Philadelphia Rod (rear)

Double faced levelling  
rod

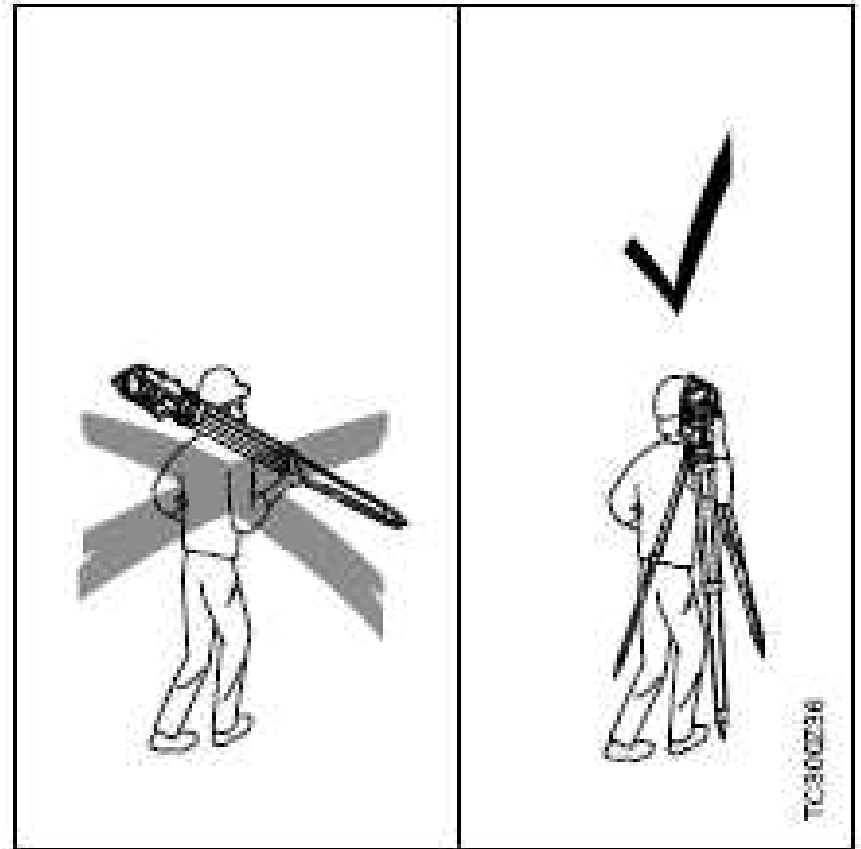
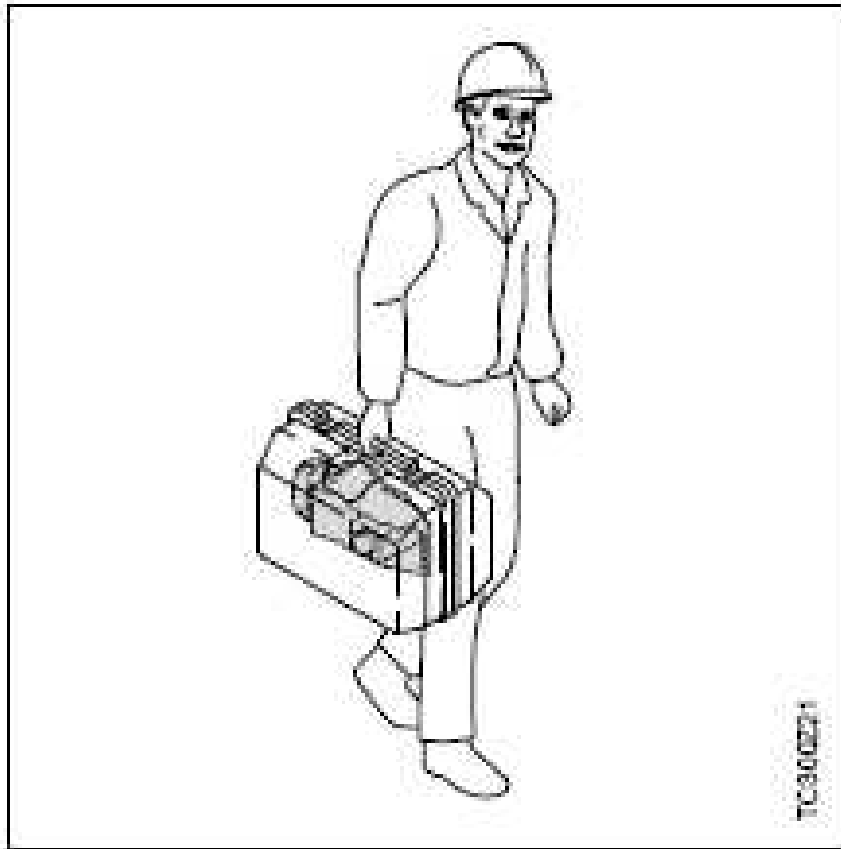
# The Equipments : Rod Level and Tripod



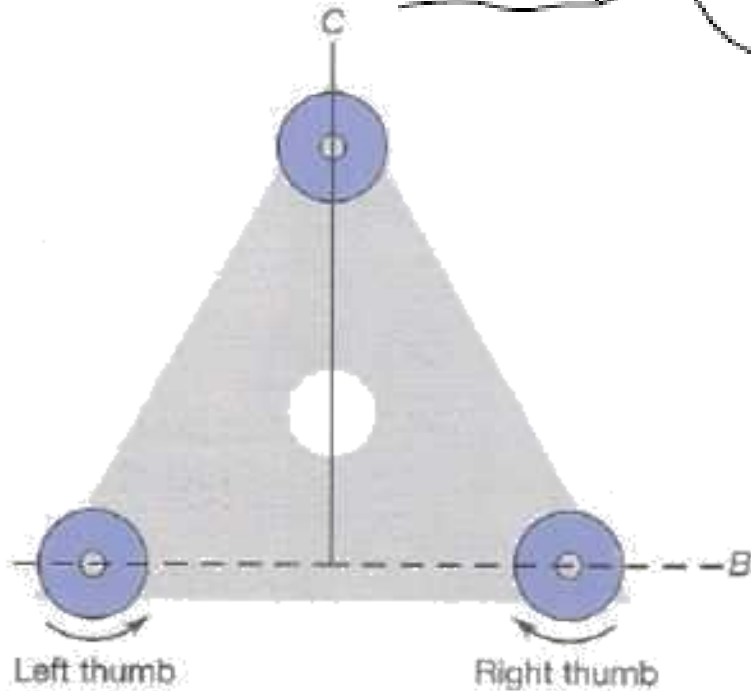
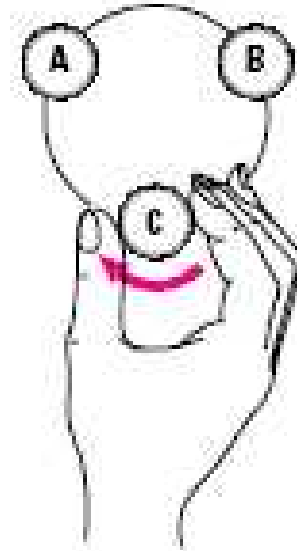
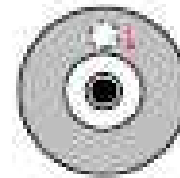
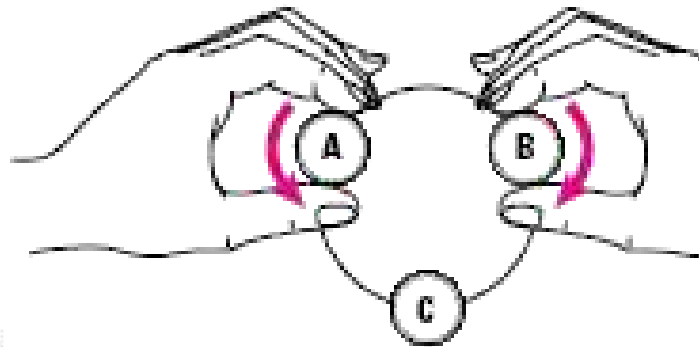
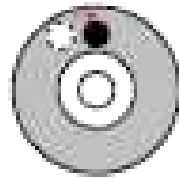
# Carrying and Setting Up a Level

- Always carry it in the container.
- Screw the head snugly on the tripod.
- For bull eye's bubble, alternately turn one screw and then the other two.
- On side-hill setups, place one leg on the uphill side and other two on the down hill side.
- Use hand level to check for proper height of the setup before precisely levelling the instrument.  
vial

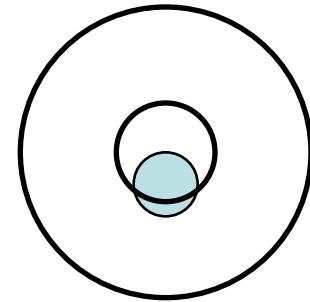
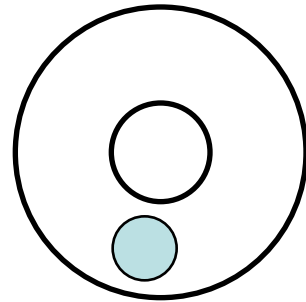
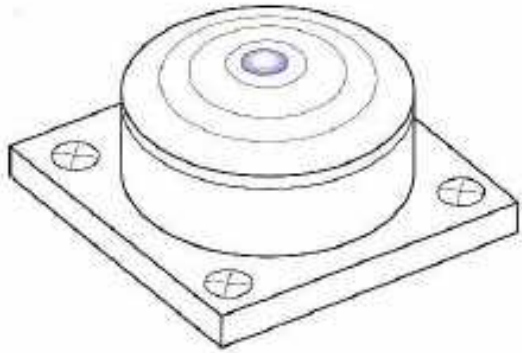
# Carrying and Setting Up a Level



# Carrying and Setting Up a Level



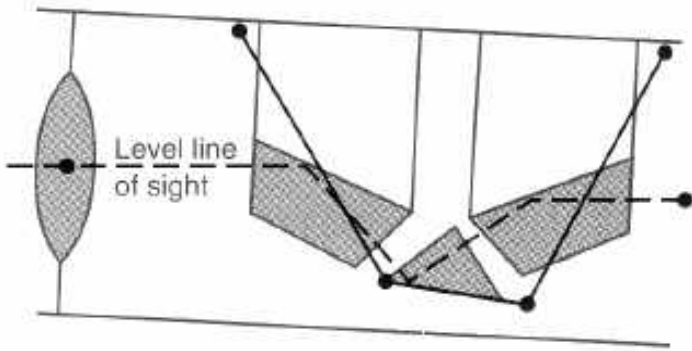
# Pond Bubble



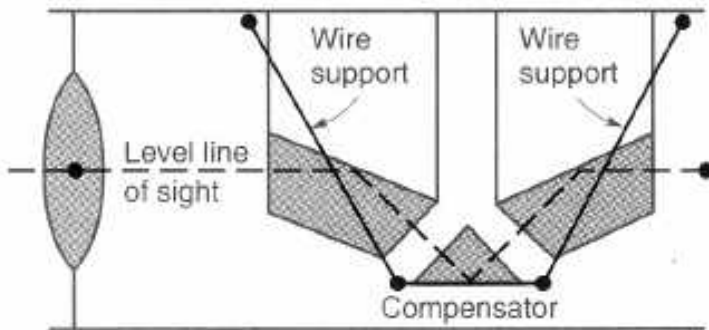
- When pond bubble is centred the instrument's standing axis is approximately vertical.
- The compensators in the instrument take over and adjust the optical Line of Collimation so that it is horizontal.
- When the instrument is rotated the compensators ensure that a **horizontal plane of collimation** is swept out.

# Automatic Levels

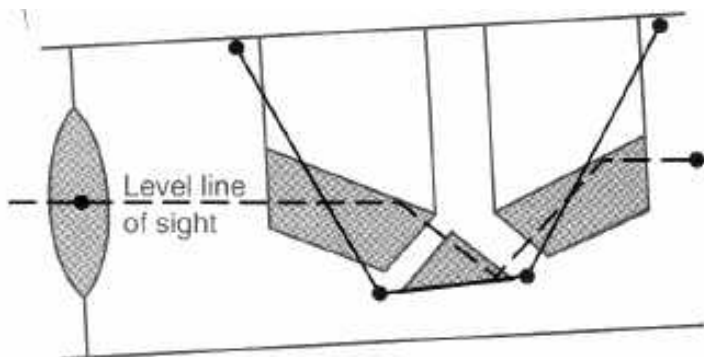
After the pond bubble is manually centered, an automatic compensator levels the line of sight, and keeps it level.



When telescope tilts up, compensator swings backward.

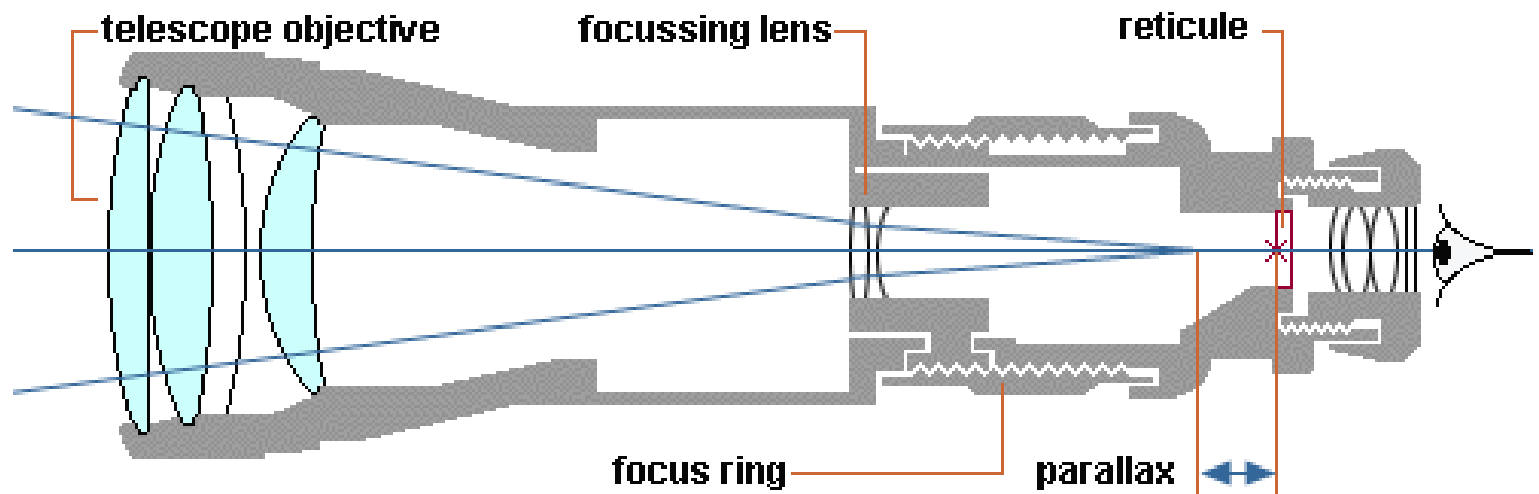
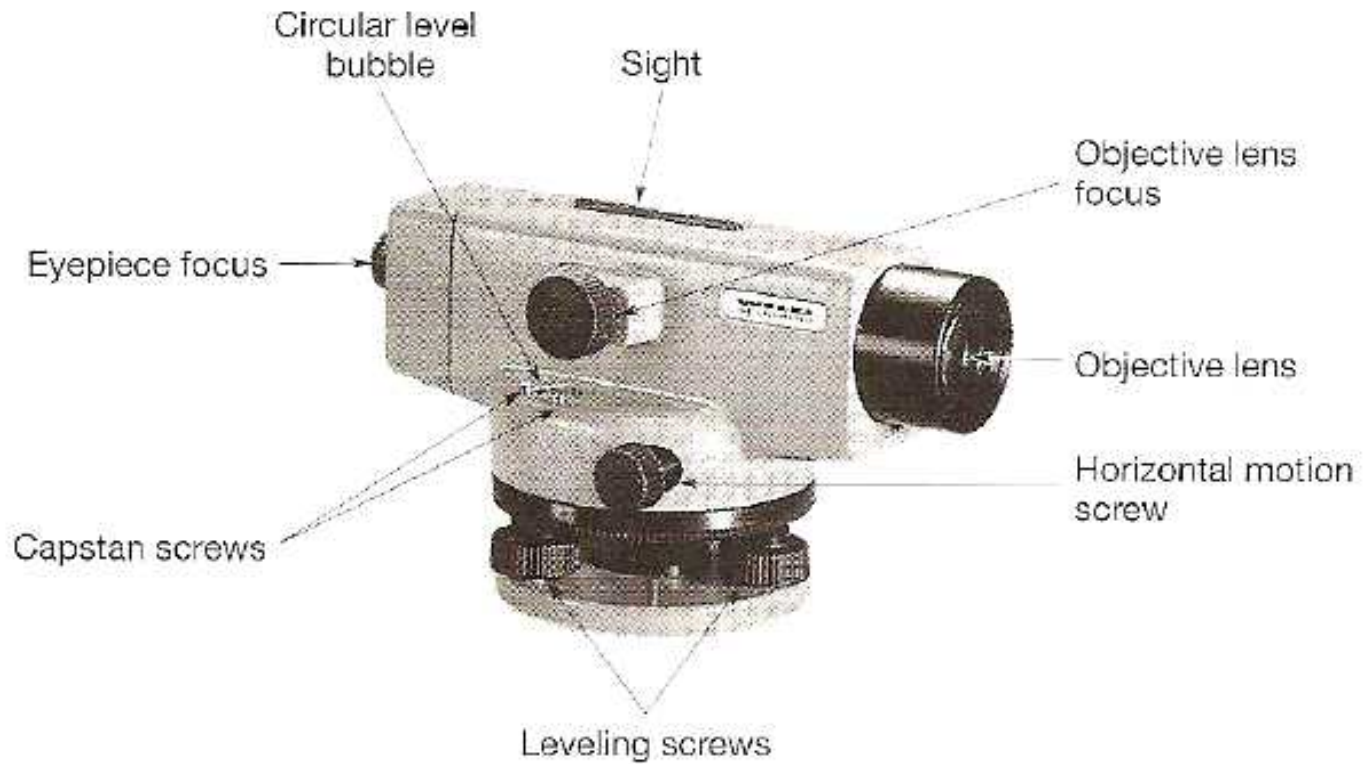


Telescope horizontal



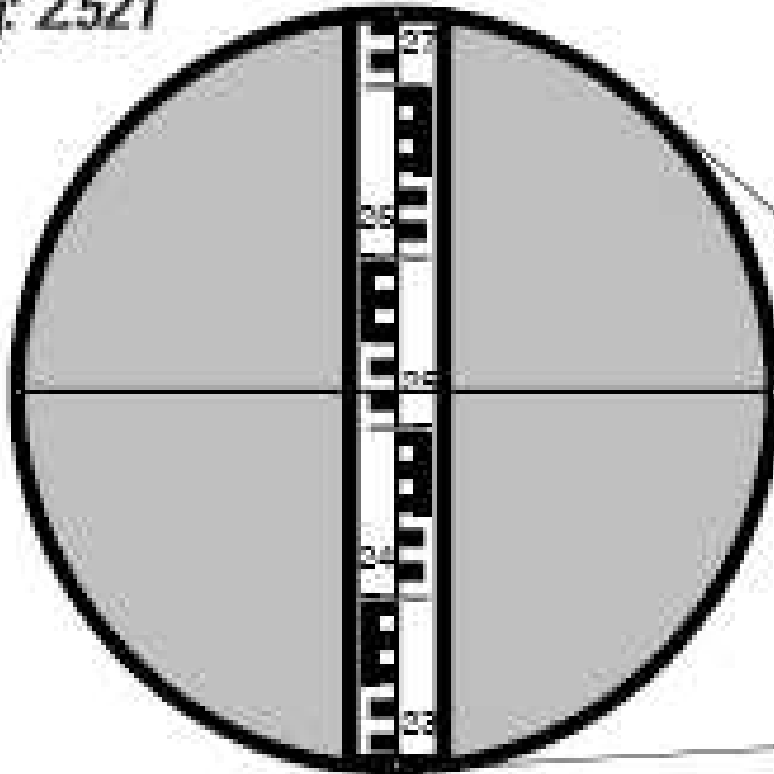
When telescope tilts down, compensator swings forward.



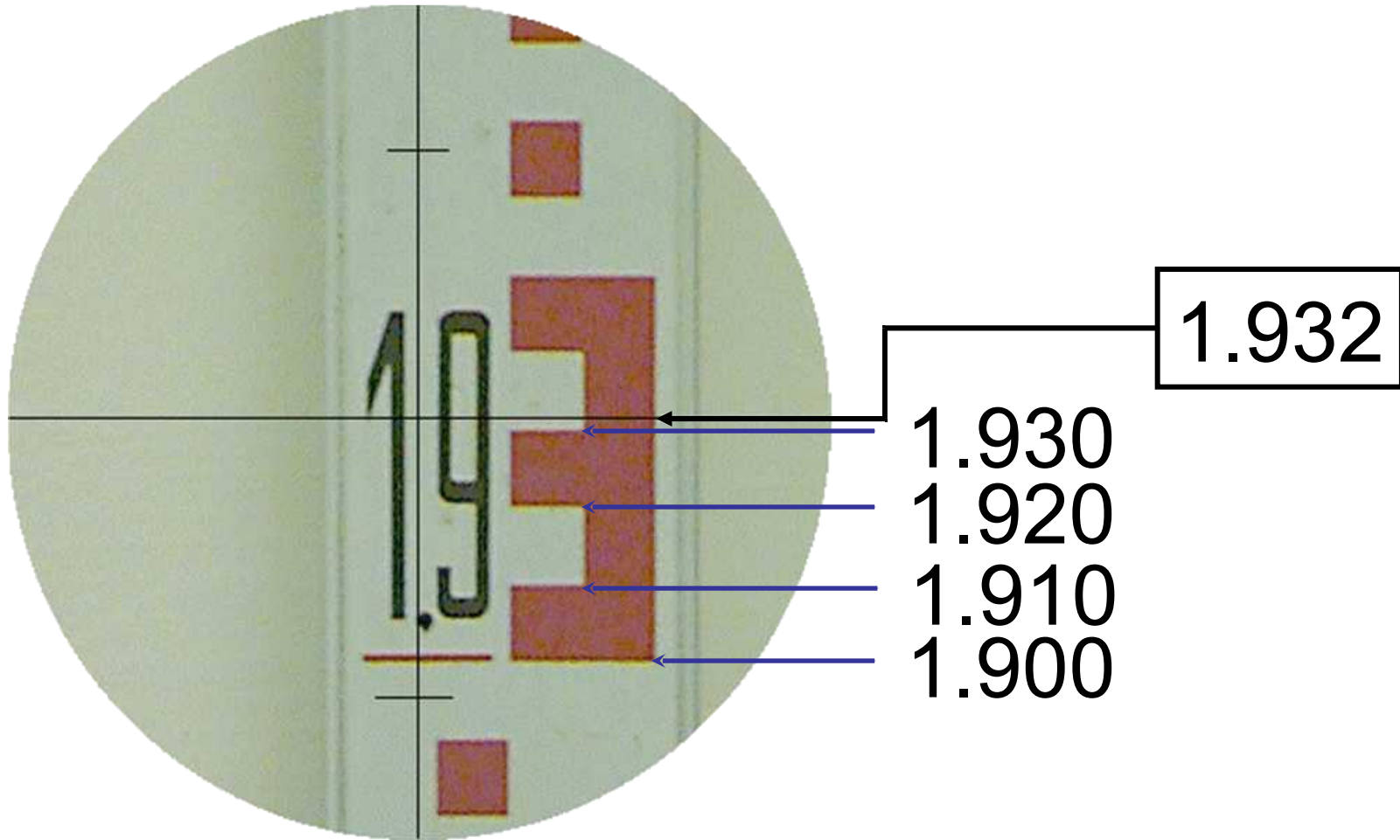


# E-type Levelling Staff (1)

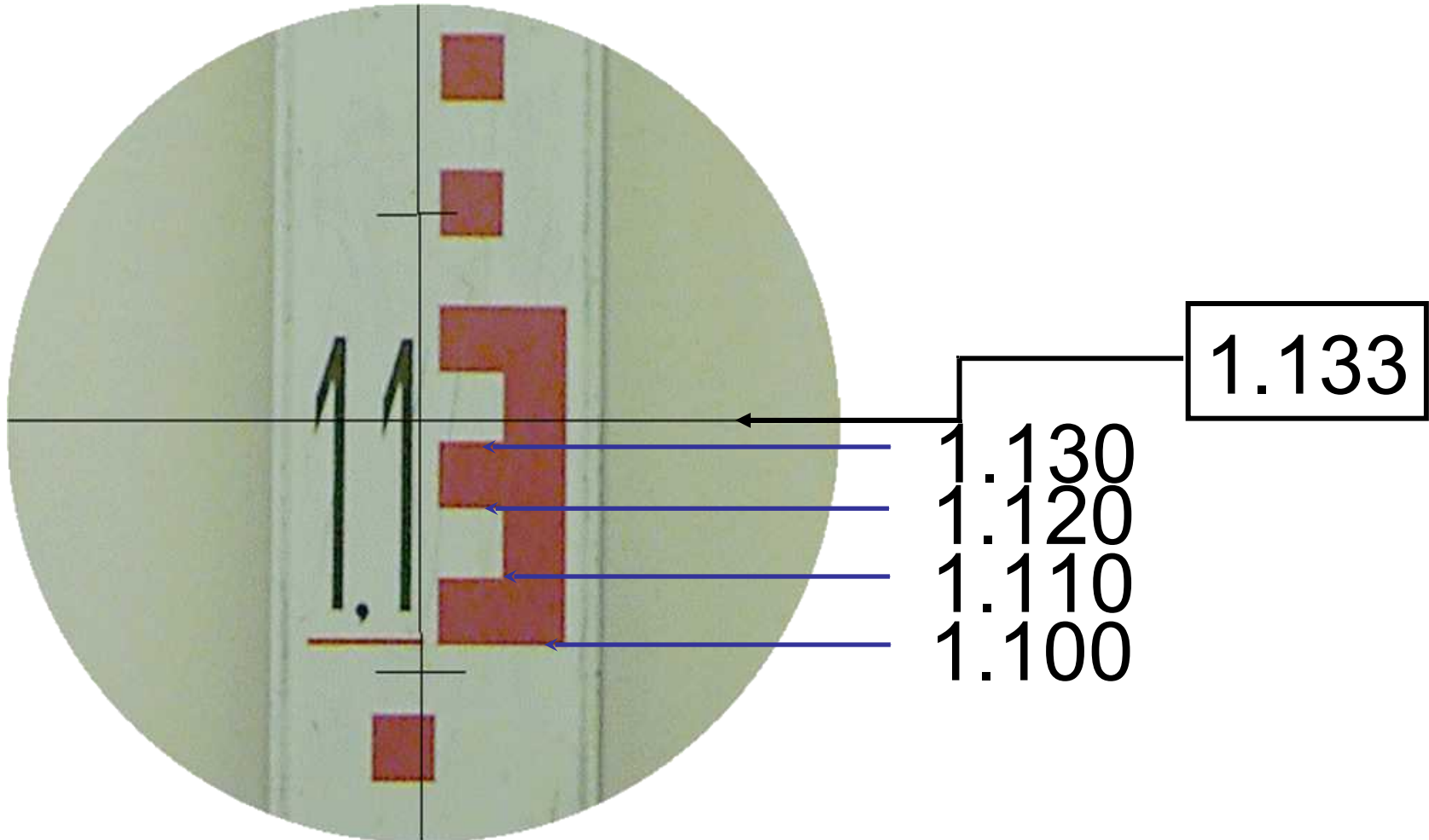
*Reading: 2521*



# E-type Levelling Staff (2)



# E-type Levelling Staff (3)



# Common Error Sources

- Level rod not vertical
- Levelling rod not fully extended or incorrect length
- Level instrument not level
- Instrument out of adjustment
- Environment - wind and heat

# Suggestions for Good Levelling

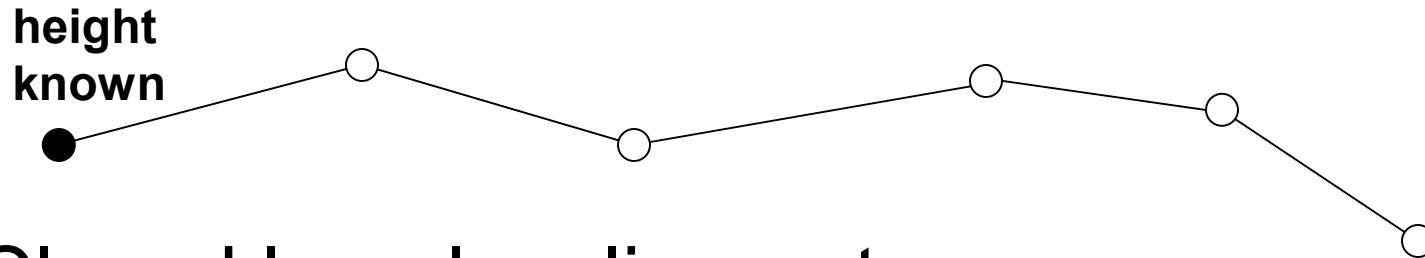
- Anchor tripod legs firmly
- Check the bubble level before and after each reading
- Take as little time as possible between BS and FS
- Try to keep the distance to the BS and the FS equal
- Provide the rod person with a level for the rod

# Elimination of Parallax

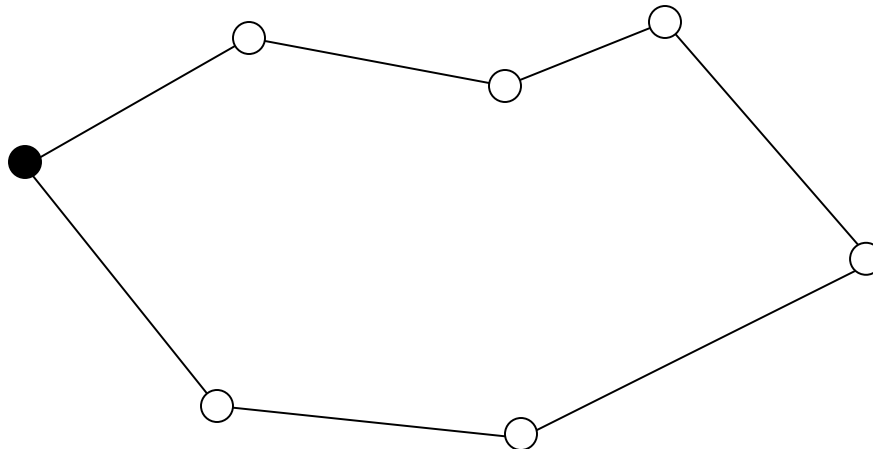
- Parallax is the apparent movement of the image produced by movement of the observer's eye at the eyepiece.
- It is eliminated by focusing the telescope on infinity and then adjusting the eyepiece until the cross-hairs appear in sharp focus. The setting will remain constant for a particular observer's eye.

# Types of Levelling Nets

- Open leveling nets



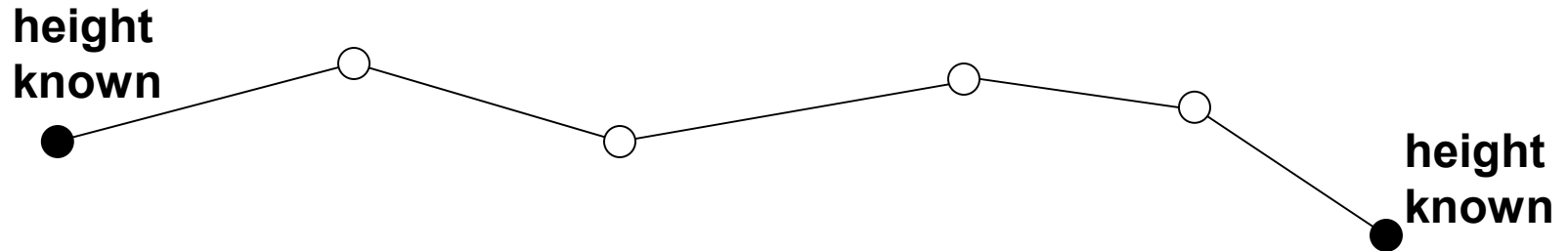
- Closed loop leveling nets





# Types of Leveling Nets

- Closed link or closed connecting leveling nets



- Leveling between two points is performed in two steps: forward ( $\Delta H_{AB}$ ) and backward ( $\Delta H_{BA}$ ). The height differences,  $\Delta H_{AB}$  and  $\Delta H_{BA}$  should be theoretically equal.

$$\forall \Delta H_{AB} - \Delta H_{BA} \leq d$$

# HPC Method

## Height of the Plane of Collimation Method

- It consists in finding the elevation of the plane of collimation for every set up of the instrument and then obtaining the reduced levels (RL) of point with reference to the respective plane of collimation.
- The HPC method is generally used in profile levelling and in setting out levels for constructional work.

# HPC Method

## Height of the Plane of Collimation Method

- Same staff position as last reading therefore the same row
- New staff position therefore a new row

$$\text{HPC} = \text{Reduced Level (RL)} + \text{BS}$$

$$\text{RL} = \text{HPC} - \text{FS}$$

$$\text{RL} = \text{HPC} - \text{IS}$$

# CLOSED-LOOP LEVELING COMPUTATION

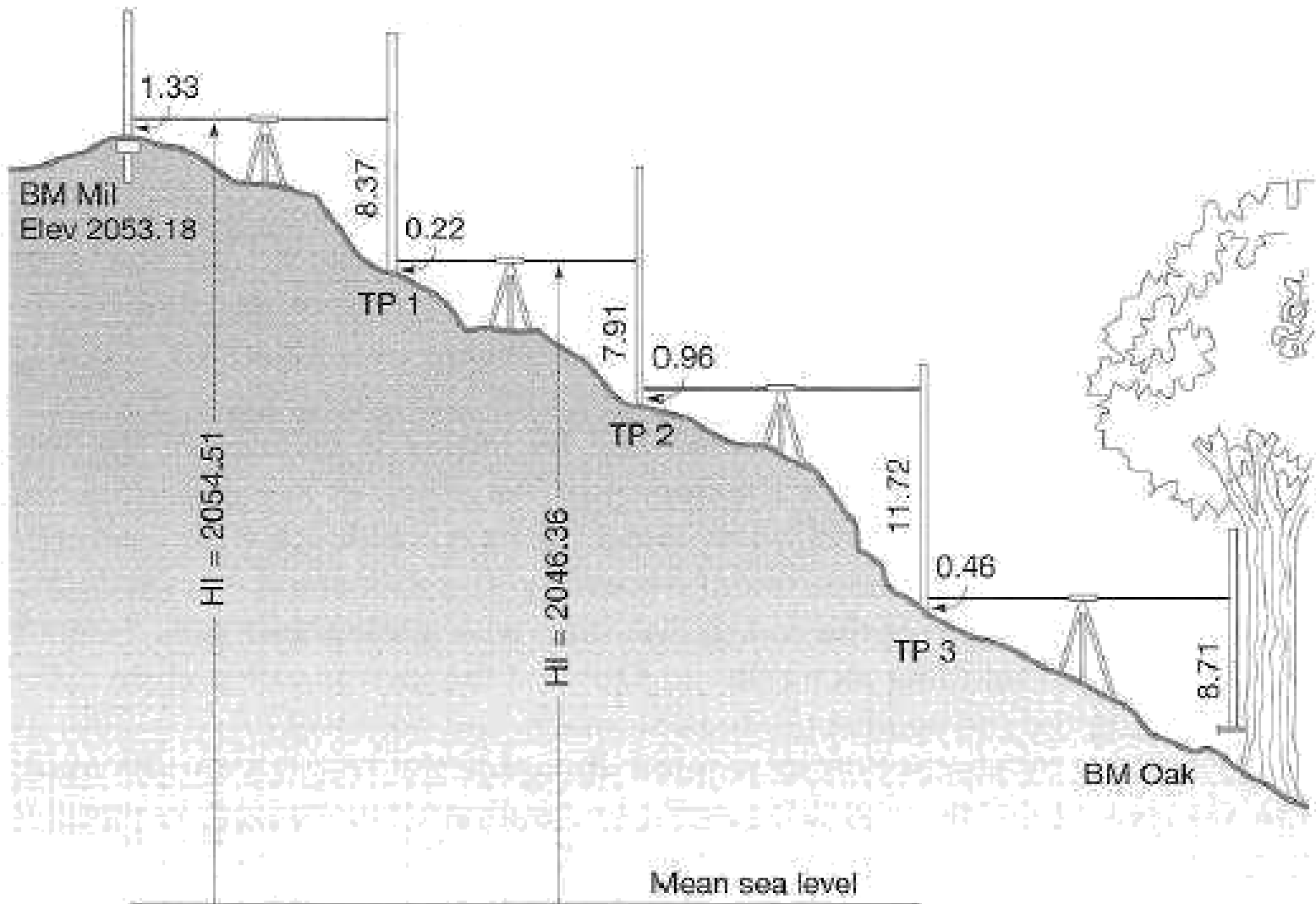
## Collimation Method

Bench Marks	Distance (m)	Rod Readings (m)			Collimation Height	Heights	Corrected Heights (m)
		BS	IS	FS			
1	40	2.147			128.621	126.474	<b>126.474</b>
2	80	1.613		0.491	129.743	128.130 <sup>-1</sup>	128.129
3	70	1.079		2.312	128.510	127.431 <sup>-2</sup>	127.429
4	50	3.452		0.286	131.676	128.224 <sup>-3</sup>	128.221
5	60	0.314		0.179	131.811	131.497 <sup>-4</sup>	131.493
6	70	0.271		3.598	128.484	128.213 <sup>-5</sup>	128.208
7	84	2.745		1.052	130.177	127.432 <sup>-6</sup>	127.426
8	42	0.682		1.423	129.436	128.754 <sup>-8</sup>	128.746
1				2.953		126.483 <sup>-9</sup>	<b>126.474</b>
$\Sigma L = 496 \text{ m}$		$\Sigma_{BS} = 12.303$			$126.483 - 126.474 = 0.009 \text{ m}$		
Check : $\Sigma_{BS} - \Sigma_{FS} = 12.303 - 12.294 = 0. = 0.009\text{m}$							

# HPC Method

## Height of the Plane of Collimation Method

<i>BS</i>	<i>IS</i>	<i>FS</i>	<i>HPC</i>	<i>RL</i>	<i>Remarks</i>
1.5	2.5		62.0	60.5	TBM (60.5) 1A
	4.0			59.5	2A
				58.0	3A
3.0	5.5	2.0	63.0	60.0	Change pt 4A (1B)
				57.5	2B
6.0		1.0	68.0	62.0	Change pt 3B (1C)
		3.0		65.0	TBM (65.1) 2C
10.5	12.0	6.0		65.0	Checks
6.0				60.5	Misclosure 0.1
4.5				4.5	<i>Correct</i>



# DIFFERENTIAL LEVELS

Sta.	B.S.	H.I.	F.S.	Elev.	Adj. Elev.
BM Mil.	1.33			2053.18	2053.18
		2054.51			
TP1	0.22		8.37	2046.14	2046.14
		2046.36	7.91		
TP2	0.96		<del>8.91</del>	2038.45	2038.44
		2039.41			
TP3	0.46		11.72	2027.69	2027.68
		2028.15			
BM Oak	11.95		8.71	2019.44	2019.42
		2031.39			
TP4	12.55		2.61	2028.78	2028.76
		2041.33			
TP5	12.77		0.68	2040.65	2040.62
		2053.42			
BM Mil.			0.21	2053.21	2053.18
	$\Sigma = +40.24$		$\Sigma = -40.21$		

Page Check:

2053.18	
+ 40.24	
<hr/>	
2093.42	
- 40.21	
<hr/>	
2053.21	Check

# GRAND LAKES UNIV. CAMPUS

BM Mil. to BM Oak

BM Mil. on GLU Campus  
 SW of Engineering Bldg.  
 9.4 ft. north of sidewalk  
 to instrument room and  
 1.6 ft. from Bldg. Bronze  
 disk in concrete flush  
 with ground, stamped "Mil"

29 Sept. 2000  
 Clear, Warm 70° F  
 T.E. Henderson N  
 J.F. King  $\phi$   
 D.R. Moore  $\pi$   
 Lietz Level #6

BM Oak is a temporary  
 project bench mark located  
 at corner of Cherry and  
 Pine Sts., 14 ft. West of  
 computer laboratory. Twenty  
 penny spike in 18" Oak  
 tree, 1 ft. above ground.

$$\text{Loop Misclosure} = 2053.21 - 2053.18 = 0.03$$

$$\text{Permissible Misclosure} = 0.02 \sqrt{n} = 0.02 \sqrt{7} = 0.05 \text{ ft.}$$

$$\text{Adjustment} = \frac{0.03}{7} = 0.004' \text{ per H.I.}$$

J.E. Henderson

# Calculation checks

## Simple check

$$\sum FS - \sum BS = 1st\ RL - Last\ RL$$

## Full check

$$\begin{aligned} \sum IS + \sum FS + \sum (RLs\ except\ first) \\ = \sum (each\ HPC\ x\ number\ of\ applications) \end{aligned}$$

## Check Misclosure

Allowable Misclosure =  $\pm 5\sqrt{N}$  mm. ("Rule of Thumb")

When calculations are checked and

if the misclosure is allowable

Distribute the misclosure.



# Rise and Fall Method

- It consist in determining the difference of level between consecutive points by comparing each point after the first with that immediately preceding it.
- The difference between their staff reading indicates a rise or a fall according as the staff reading at the point is smaller or greater than that preceding point.
- The RL of each point is then found by adding rise or subtracting fall to or from the RL.
- The RF method provides complete check on IS.
- It is commonly used for differential levelling.

# CLOSED-LOOP LEVELING COMPUTATION

## Rise & Fall Method

Bench Marks	Distance (m)	Rod Readings (m)			DH (m)		Heights (m)
		BS	IS	FS	Rise (+)	Fall (-)	
1		2.147					<b>126.474</b>
	40				1.656 <sup>-1</sup>		
2		1.613		0.491		0.699 <sup>-1</sup>	128.129
	80						
3		1.079		2.312			127.429
	70				0.793 <sup>-1</sup>		
4		3.452		0.286			128.221
	50				3.273 <sup>-1</sup>		
5		0.314		0.179			131.493
	60					3.284 <sup>-1</sup>	
6		0.271		3.598			128.208
	70					0.781 <sup>-1</sup>	
7		2.745		1.052			127.426
	84				1.322 <sup>-2</sup>		
8		0.682		1.423			128.746
	42					2.271 <sup>-1</sup>	
1				2.953			<b>126.474</b>
$\Sigma L = 496 \text{ m}$		$\Sigma_{BS} = 12.303$			$\Sigma_{FS} = 12.294$	$\Sigma = 7.044$	$\Sigma = 7.035$
Check :		$\Sigma_{BS} - \Sigma_{FS} = 12.303 - 12.294 = 0.009$			$\Sigma_{RISE} - \Sigma_{FALL} = 7.044 - 7.035 = 0.009$		

# Rise and Fall Method

<i>BS</i>	<i>IS</i>	<i>FS</i>	<i>Rise</i>	<i>Fall</i>	<i>RL</i>	<i>Distance</i>	<i>Remarks</i>
1.5					60.5	0	TBM (60.5) 1A
	2.5			1.0	59.5	30	2A
	4.0			1.5	58.0	50	3A
3.0		2.0	2.0		60.0	70	CP 4A (1B)
	5.5			2.5	57.5	95	2B
6.0		1.0	4.5		62.0	120	CP 3B (1C)
		3.0	3.0		65.0	160	TBM (65.1) 2C
10.5		6.0	9.5	5.0	65.0		Checks
6.0			5.0		60.5		Misclosure 0.1
4.5			4.5		4.5		<i>Correct</i>

# Rise and Fall Method

$$\text{Fall} = \text{FS} - \text{BS}$$

$$\text{Fall} = \text{IS} - \text{BS}$$

$$\text{Rise} = \text{BS} - \text{FS}$$

$$\text{RL} = \text{Previous RL} + \text{Rise}$$

$$\text{RL} = \text{Previous RL} - \text{Fall}$$

Left half

Right half

Title  
&  
location

Page number

Weather

Party names  
&  
party jobs

Equipment  
&  
equipment ID

Data

Error checks

Sketch

Equations

Benchmark description and any other notes.

Signature



Rod holder



Note keeper



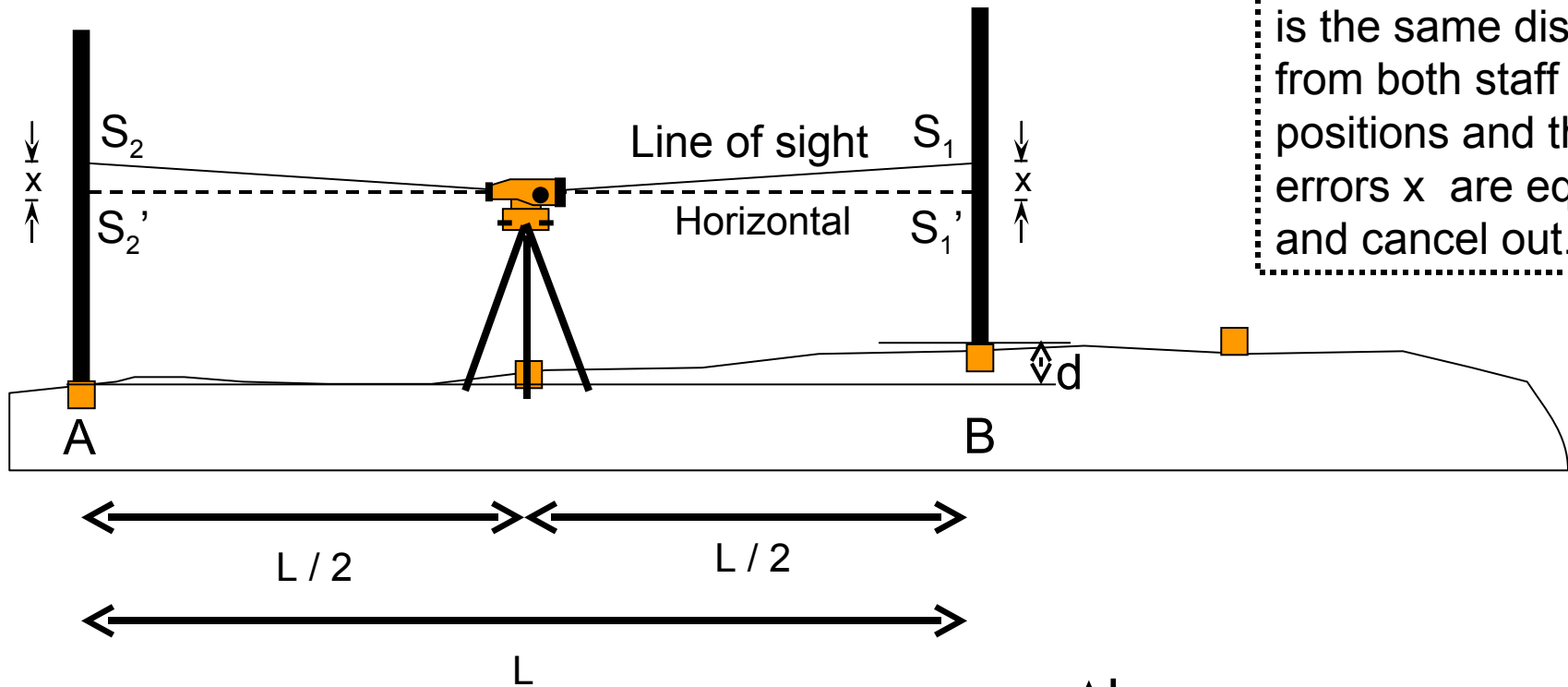
Chaining crew



Instrument reader

# Collimation Error

## Two Peg Test



Since the instrument is the same distance from both staff positions and the errors  $x$  are equal and cancel out.

The **true** height difference,  $\Delta h_T$

The **visible** height difference,  $\Delta h_A$

$$S_1 = S_1' + x \quad \text{and} \quad S_2 = S_2' + x$$

$$\Delta h_T = S_1' - S_2'$$

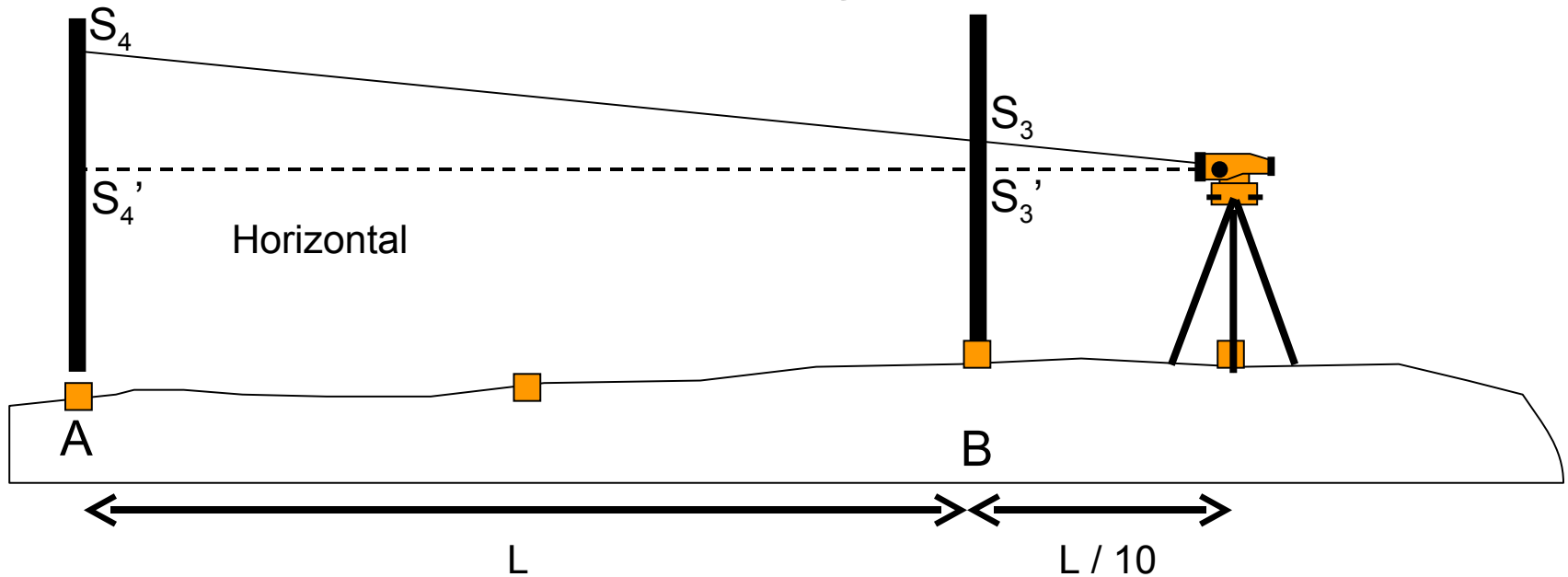
$$\Delta h_A = S_1 - S_2$$

$$\Delta h_A = (S_1' + x) - (S_2' + x)$$

$$\Delta h_A = S_1' - S_2' = \Delta h_T$$

# Collimation Error

## Two Peg Test



The **visible** height difference

$$\Delta h_A = S_3 - S_4$$

But the **true** height difference

$$\Delta h_T = S_1 - S_2$$

If  $\Delta h_A = \Delta h_T$  then the instrument is OK

If NOT then the error is  $\epsilon = (S_1 - S_2) - (S_3 - S_4) / L$  mm / m

# Collimation Error

## Two Peg Test

- Place two pegs about  $L = 30\text{m}$  (to  $40\text{m}$ ) apart.
- Set up level midway between the two pegs.
- Read staff on each peg, and calculate true height difference .
- Move level about  $L / 10 = 3\text{m}$  (or  $4\text{m}$ ) beyond one of the pegs.
- Read staff on each peg again, and calculate height difference.

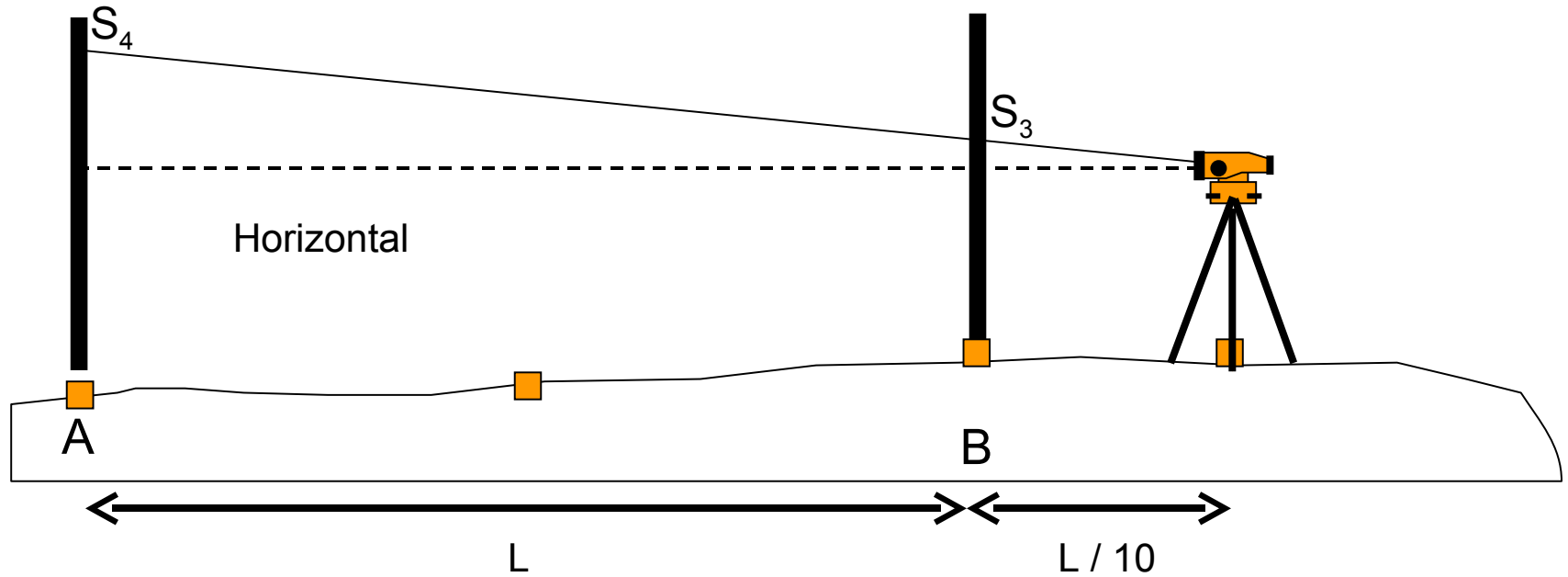
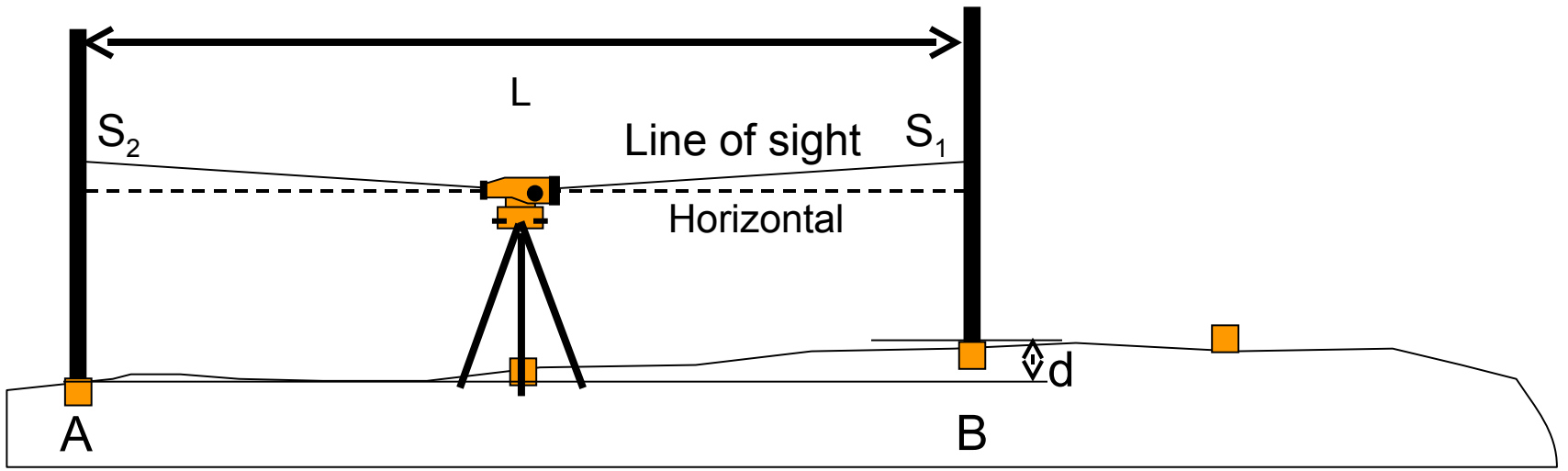
Collimation Error,  $\varepsilon$  = difference in the differences and is expressed as a number of mm per L m



# Collimation Error

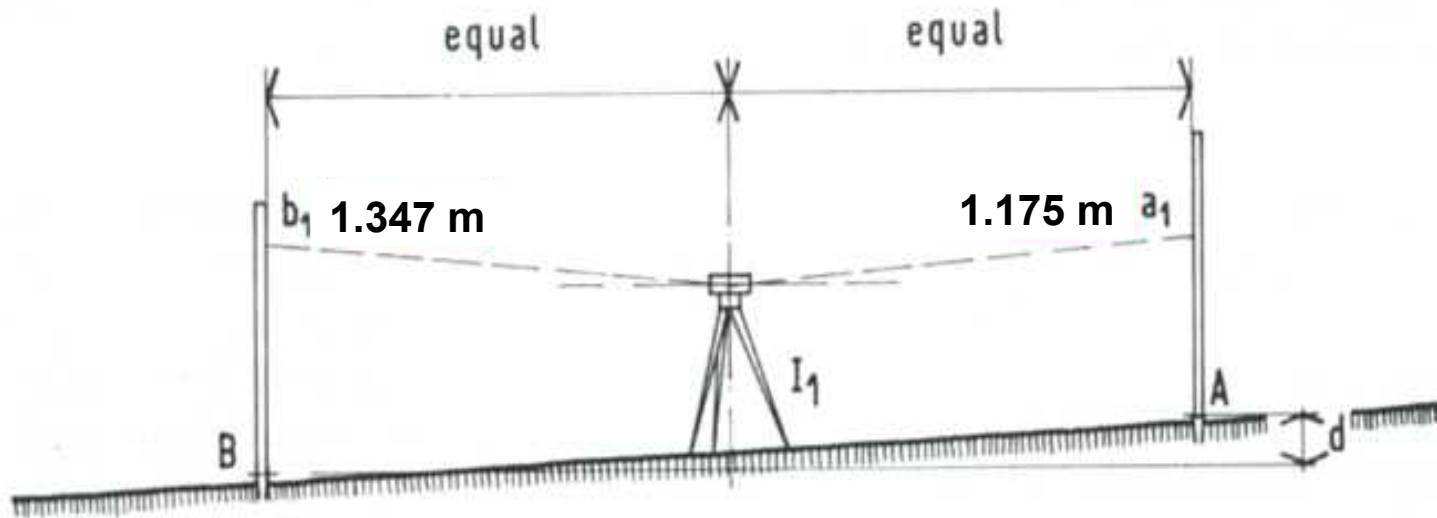
## Two Peg Test

- Acceptable errors
  - Uren and Price 1mm per 20m
  - Wimpey 4mm per 50m
- Test should be carried out regularly.

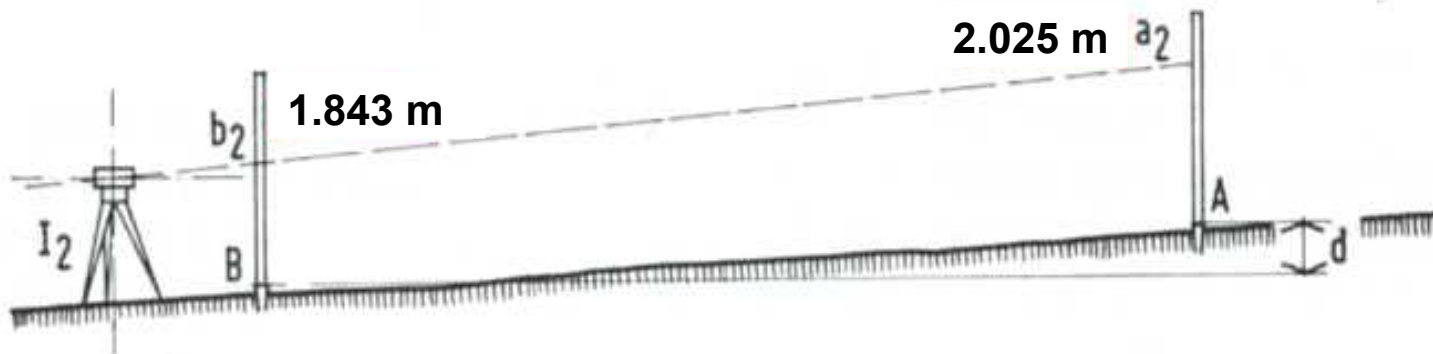


Collimation error,  $\varepsilon = (S_1 - S_2) - (S_3 - S_4) / L$ , mm/m

**A two peg test gave the following results:**



The true difference  $d = (b_1 - a_1)$ .  
If  $(a_2 - b_2) = (b_1 - a_1)$  there is  
no collimation error.



True difference between points A & B:

$$\Delta h_T = b_1 - a_1$$

$$\Delta h_T = 1347 - 1175$$

$$\Delta h_T = 172 \text{ mm}$$

Apparent difference between points A & B:

$$\Delta h_A = a_2 - b_2$$

$$\Delta h_A = 2025 - 1843$$

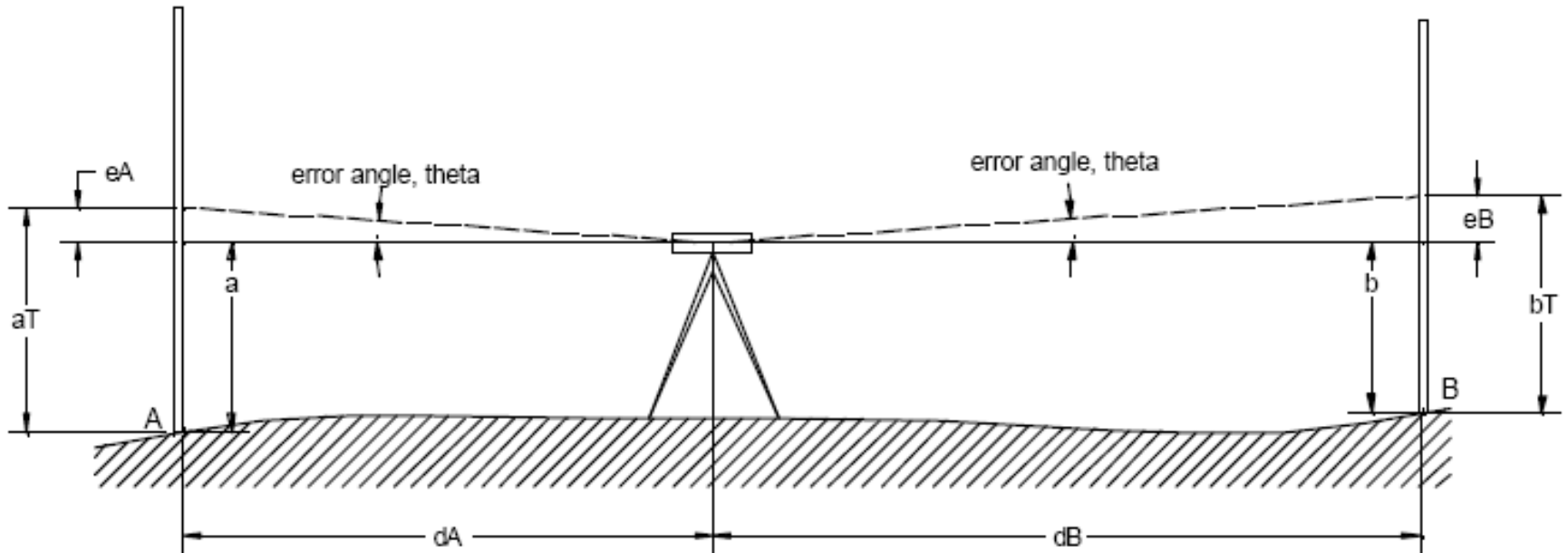
$$\Delta h_A = 182 \text{ mm}$$

If the distance between the two pegs is 30 m and the error is 10 mm (182-172) then

the **collimation error**,  $\varepsilon$  is 10 mm per 30 m

the collimation correction is  $0.01 \div 30 = 0.00033 \text{ m/m}$

# Peg Test [2]



$d_A$  = length of shot to A

$d_B$  = length of shot to B

$a$  = actual reading at A

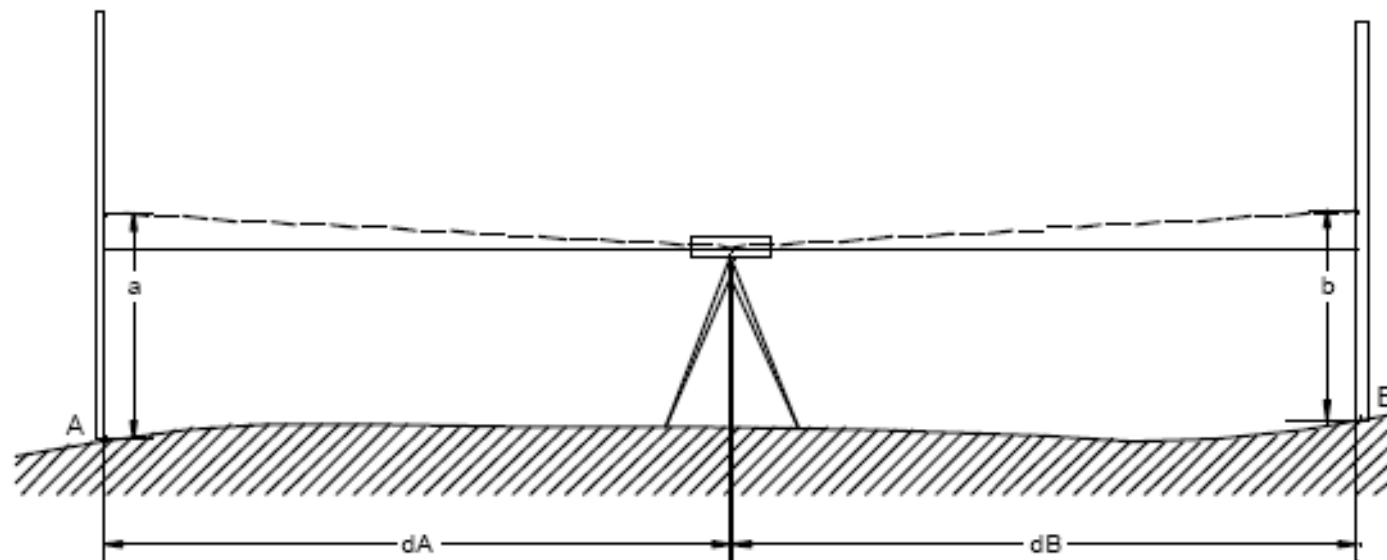
$b$  = actual reading at B

$e_A$  = error component at A due to telescope misalignment

$e_B$  = error component at B due to telescope misalignment

$a_T$  = reading at A seen through telescope

$b_T$  = reading at B seen through telescope



$$a = a_T - e_a$$

and

$$b = b_T - e_b$$

$$e_a = d_A \tan \theta$$

and

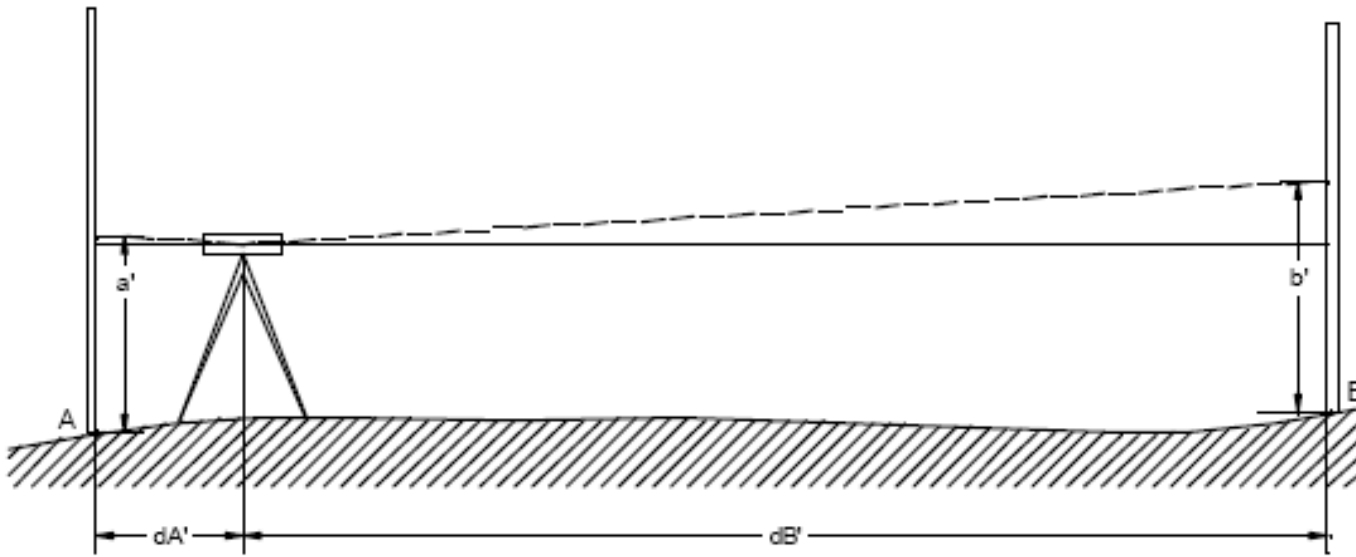
$$e_b = d_B \tan \theta$$

$$a - b = [a_T - d_A \tan \theta] - [b_T - d_B \tan \theta]$$

$$a - b = (a_T - b_T) + (d_B - d_A) \tan \theta$$

therefore :

$$a - b = a_T - b_T \quad \text{only if } [\tan \theta = 0] \text{ and/or } [d_B - d_A] = 0$$



$$a - b = (a' - b') + (d_{B'} - d_{A'}) \tan \theta$$

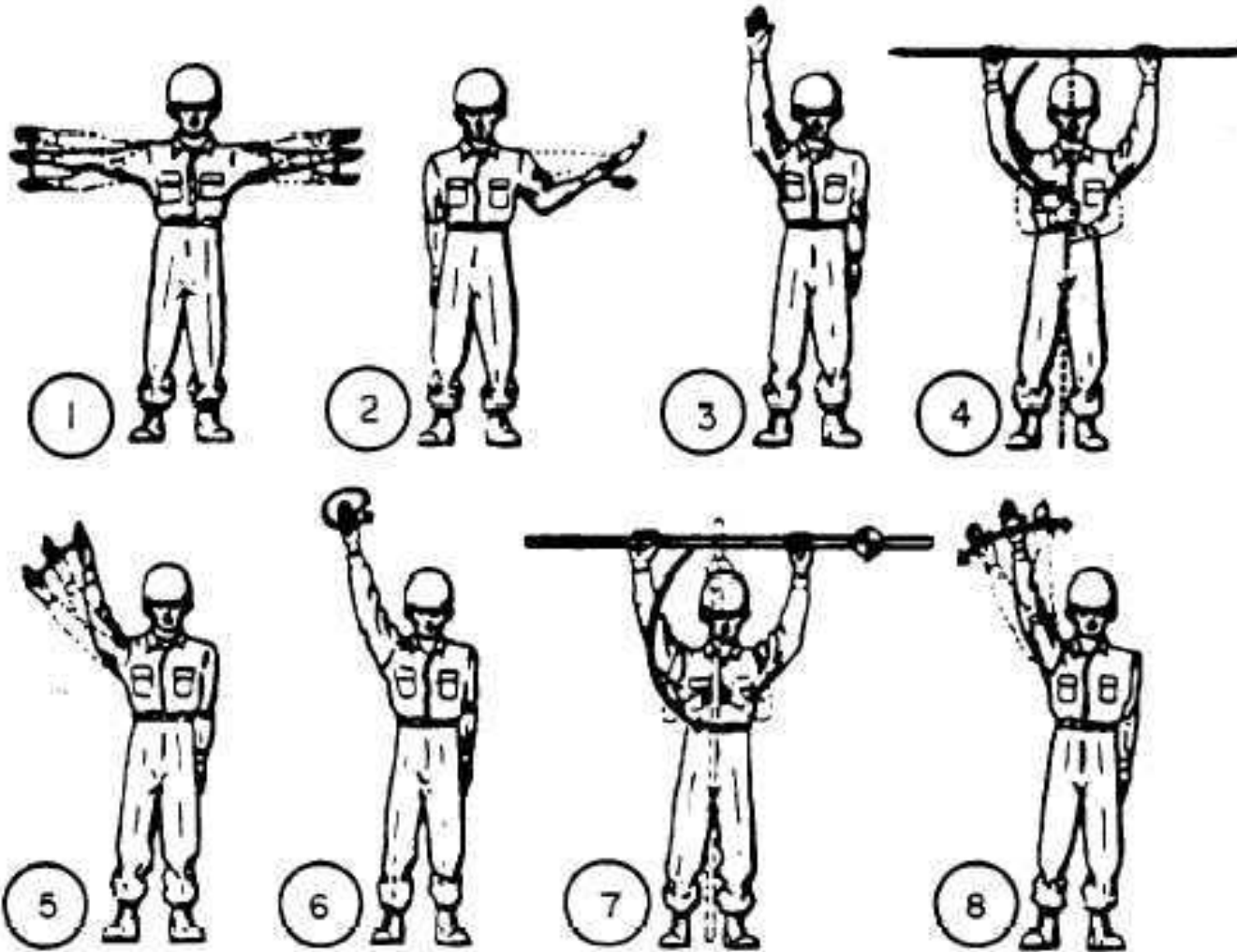
$$\tan \theta = \frac{(a - b) - (a' - b')}{(d_{B'} - d_{A'})}$$

$$\theta = \tan^{-1} \left[ \frac{(a - b) - (a' - b')}{(d_{B'} - d_{A'})} \right]$$

Peg Test results are reported in degrees and as slopes. A useful way to express peg test results is in the units of

“\_\_ mm (high/low) per m length of shot”.

# Surveyor's Hand Signals (1)

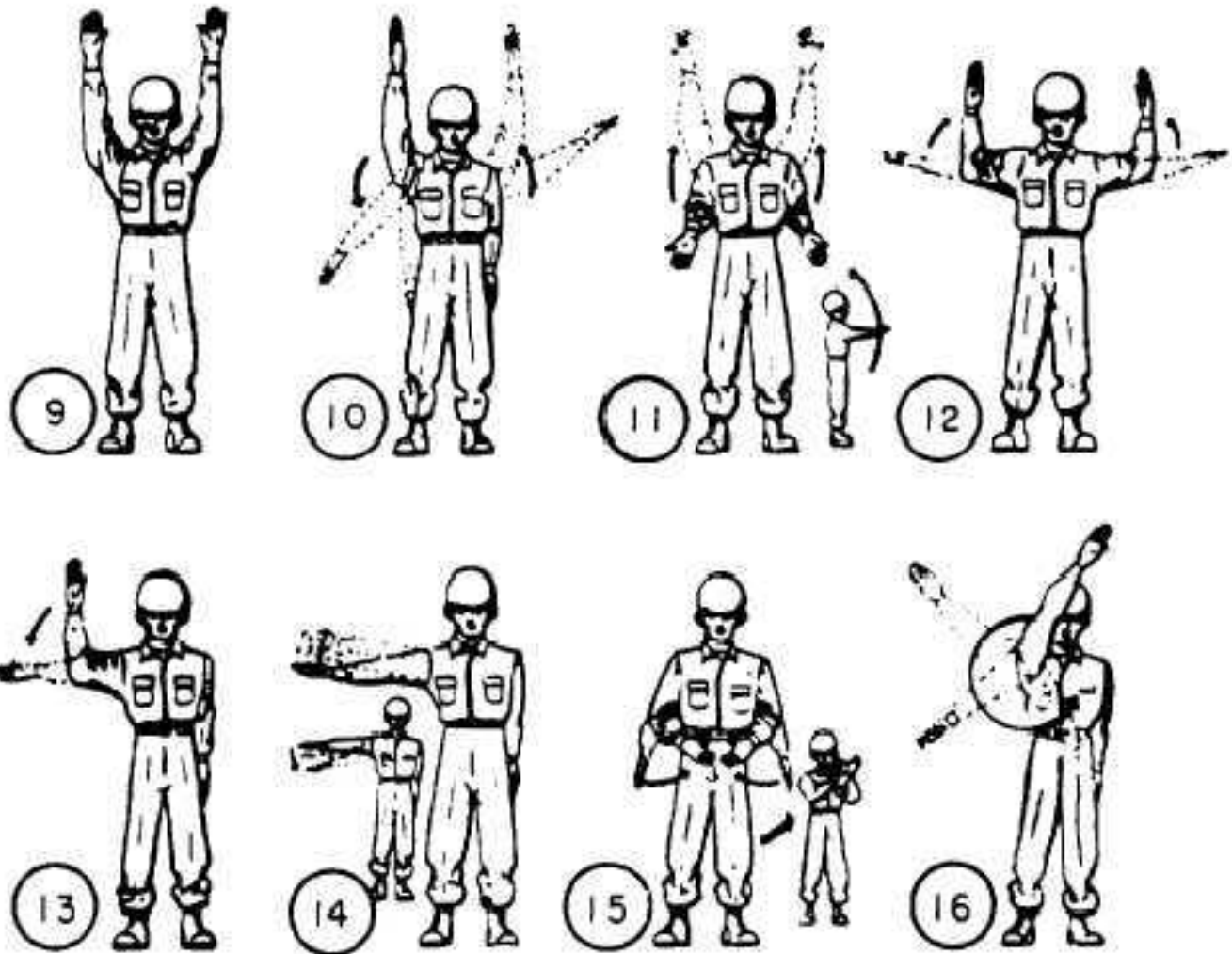




# Surveyor's Hand Signals <sup>(2)</sup>

1. All right.
2. Move right or left.
3. Give me a back sight.
4. Give me a line or this is a hub.
5. Plumb the rod.
6. Establish a turning point.
7. This is a turning point.
8. Wave the rod.

# Surveyor's Hand Signals (3)



# Surveyor's Hand Signals (4)

1. Face the rod.
2. Reverse the rod.
3. Boost the rod.
4. Move forward.
5. Move back.
6. Up or down.
7. Pick up the instrument.
8. Come in.